



# Testing an Optimization Model for Optimal Sewer Layout and Wastewater Treatment Locations

Faisal M. Alfaisal<sup>1</sup>

<sup>1</sup> Department of Civil Engineering, King Saud University,  
Riyadh, Saudi Arabia

Email: [falfaisal@ksu.edu.sa](mailto:falfaisal@ksu.edu.sa)



# Overview

- Introduction.
- Cost Functions.
- Methodology.
  - Connectivity Model.
- Two Example Systems.
- Results.
- Conclusion and Recommendations.

# Introduction

- **Optimization model to minimize total costs of sewer layout and WWTPs locations, taking into consideration mass balance and energy constraints.**
- **The Iso-nodal Line (INL) concept is used for solving water collection/branched system problems.**
- **The application of the model is illustrated through two simple examples and the results are discussed.**

# Literature review

## Cost Functions

Reference	Overall cost
<b>Mays et al. (1983)</b>	<p>= <math>2.88Q^{0.99}</math> (Capital cost of WWTP).            = <math>0.0825Q^{0.96}</math> (Operation and maintenance costs of WWTP).            = <math>80Q^{0.461}</math> (Capital cost of pipeline).            = <math>4.56 \times 10^{-3} * \text{distance(mi)} * Q^{0.495}</math> (operation and maintenance costs of pipeline).            * All flow rates Q are in gallons per day</p>
<b>Al-A'ama and Nakhla (1995)</b>	<p>= <math>2.03\\$/m^3</math>            The cost included capital cost (= <math>1.33 \text{ US}\\$/m^3</math>),            tertiary treatment (= <math>0.16 \text{ US}\\$/m^3</math>),            collection (= <math>0.3 \text{ US}\\$/m^3</math>)            and distribution (= <math>0.06 \text{ US}\\$/m^3</math>).</p>
<b>Brand and Ostfeld (2011)</b>	<p>= <math>0.33\\$/m^3</math> (capital costs of WWTP)</p>
<b>Kajenthira et al. (2012)</b>	<p>Secondary TWW in the range of <math>0.13\text{--}0.63 \text{ US}\\$/m^3</math>,            Tertiary TWW in the range of <math>1.19\text{--}2.03 \text{ US}\\$/m^3</math>.</p>
<b>Al-Zahrani et al. (2016)</b>	<p>TWW reuse ranges from <math>0.82</math> to <math>2.03 \text{ US}\\$/m^3</math>            with an average cost of <math>1.43 \text{ US}\\$/m^3</math>.</p>

# Connectivity Model

Example:

- 3- INL (i,j,k).
- 2 nodes in each INL ( $i_1, i_2; j_1, j_2; k_1, k_2$ ).
- Possible Layouts to INL (k) are Layout 1, Layout 2, and Layout 3.

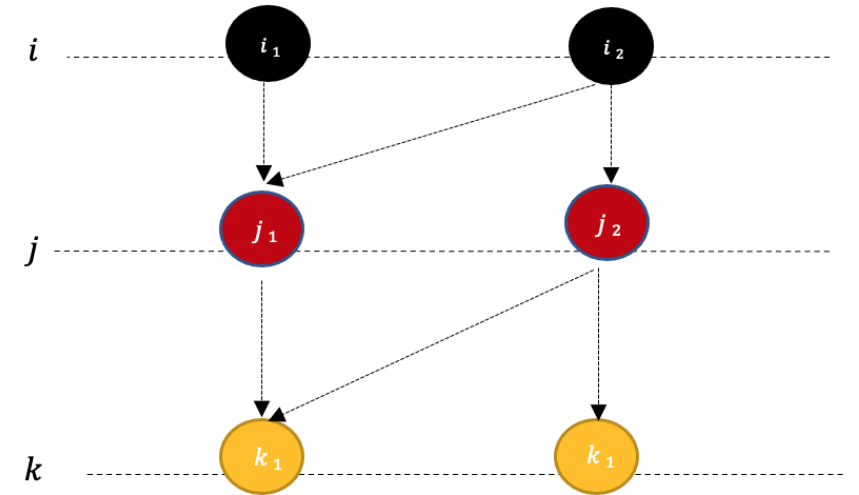
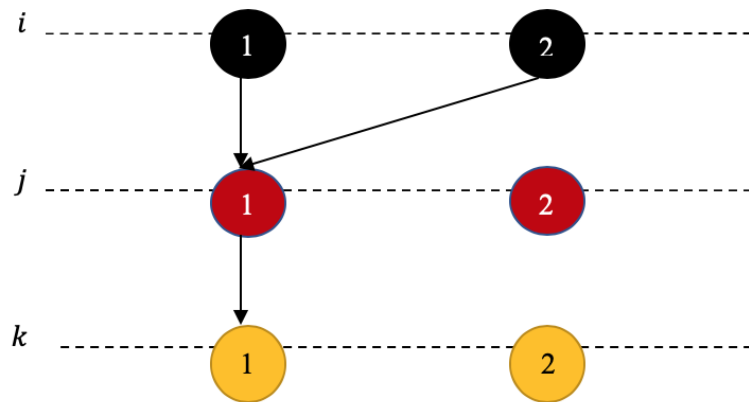
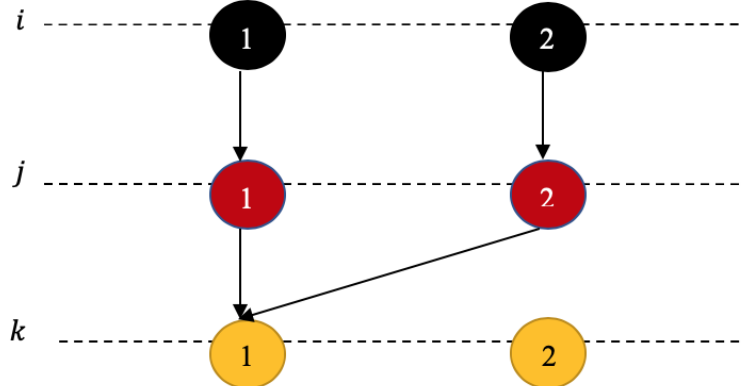


Fig 1. Possible connectivity lines at Isondal line  $i$  and  $j$ .

Layout 1



Layout 2



Layout 3

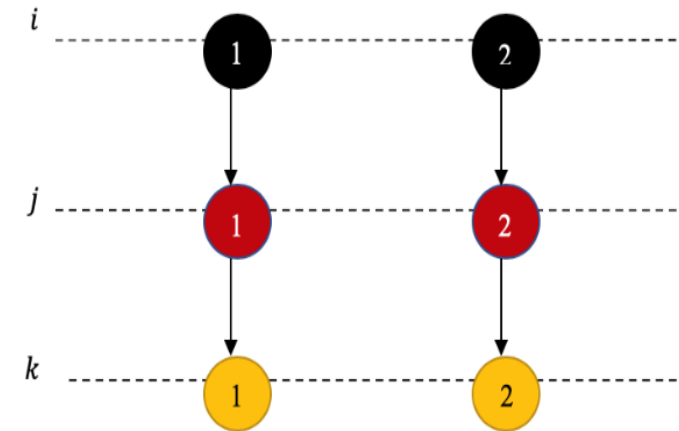


Fig 2. Possible Layouts to Isondal line  $k$ .

# Mathematical Formulation

- The mathematical formulation contains of objective function, constraints, decision variables;
  - Objective function which is minimizing the total costs of size, type, and location of sewer pipe and WWTP.
  - Subject to; Connectivity model, Continuity equations, Hydraulic equations constraints.
  - Variables are discharges that are bounded between lower and upper values.
- The model is formulated using MINLP in GAMS, solved by the BARON solver.

# Test Example 1

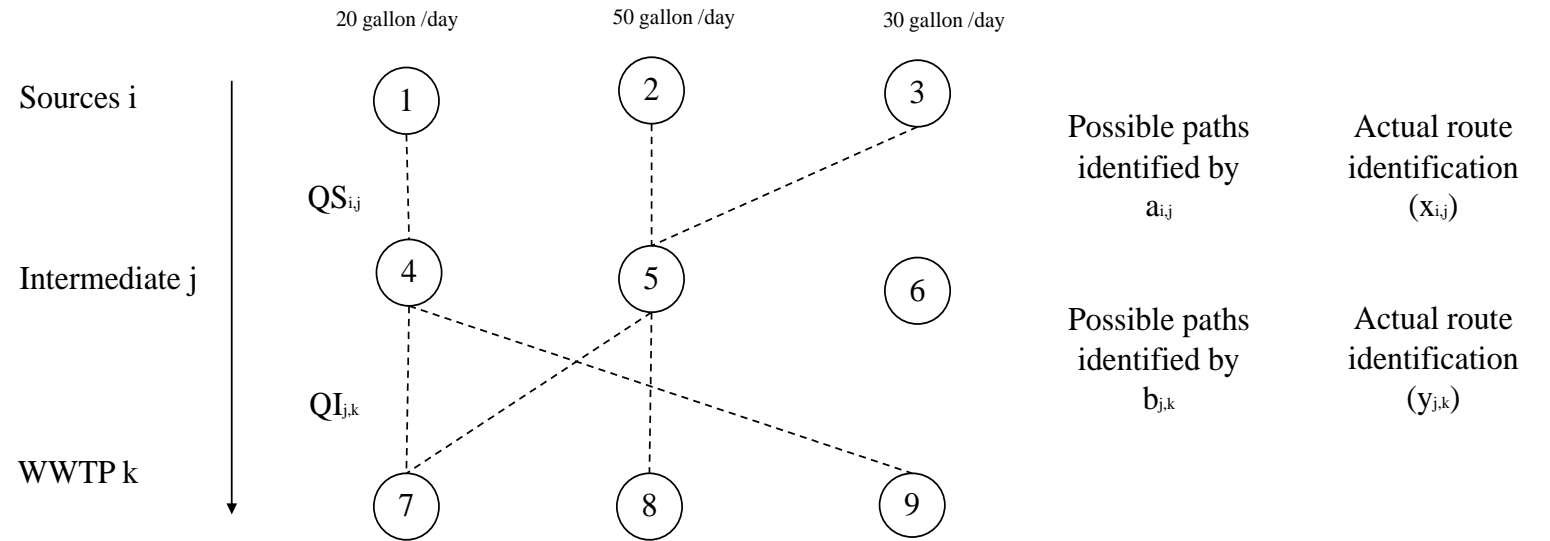


Fig 3. Input Values for the Model in GAMS for Example 1

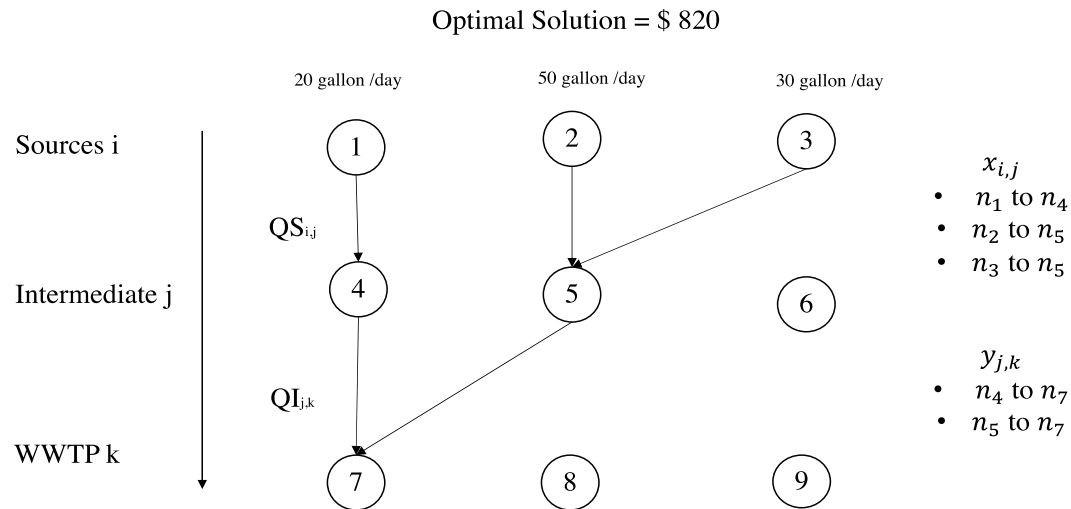


Fig 4. The Optimum Configuration for Example 1

# Test Example 2

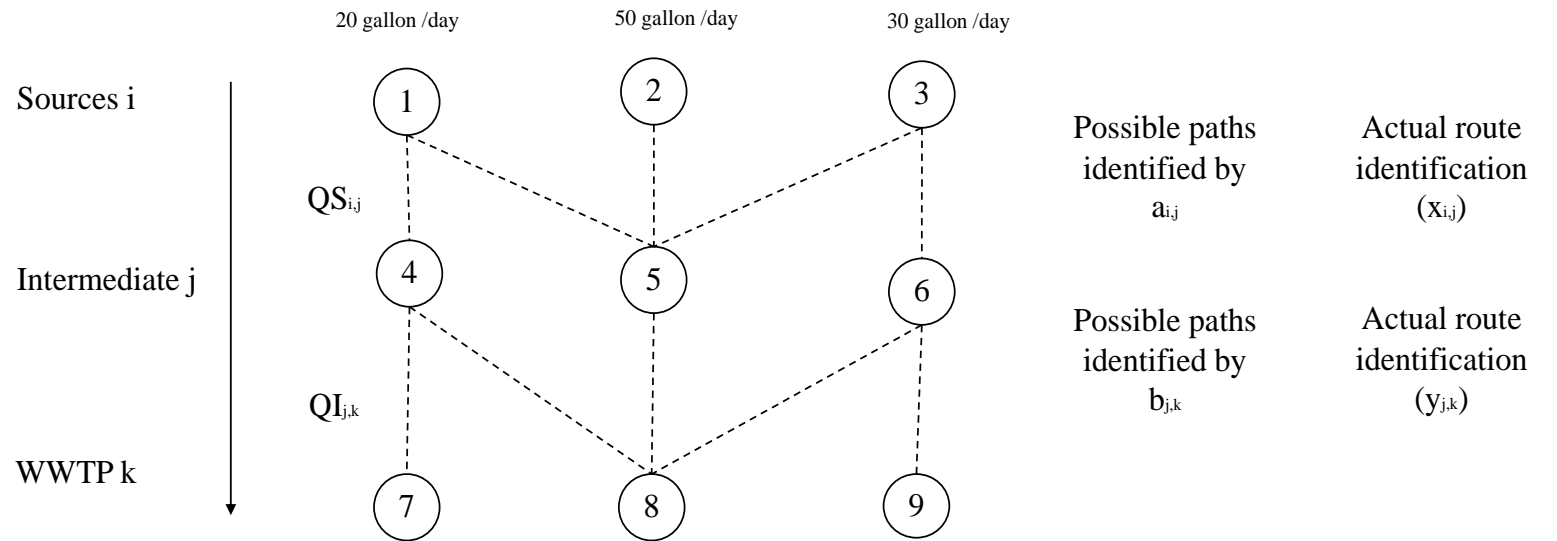


Fig 5. Input Values for the Model in GAMS for Example 2

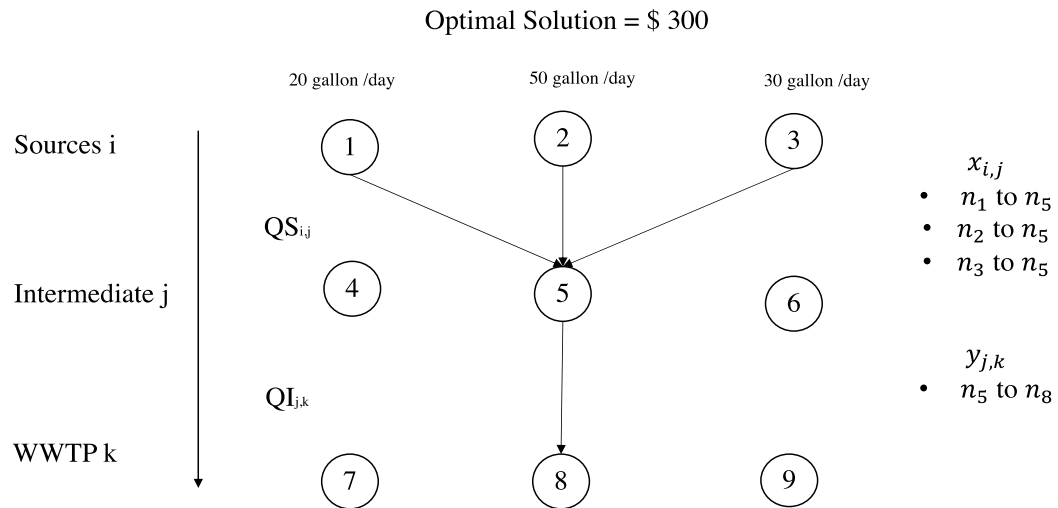


Fig 6. The Optimum Configuration for Example 2



# Conclusion and Recommendations

- Wastewater systems are very expensive so developing such novel approaches [[Connectivity model](#)] would help to minimize the total costs.
- The simple scenario demonstrates that using the method allows for significant cost saving for large systems while [further testing](#) and [developments](#) may be needed.
- The methodology minimizes the total costs without considering [the capacity limitation](#) of a WWTP.
- The methodology is beneficent the [planning](#) and [designing](#) regional/local wastewater systems.
- Hydraulic constraints including [Pipe diameter](#) and [Lifting station](#) can be added to consider commercial diameter and total dynamic head in the system simultaneously. **(Alfaisal and Mays,2021)**

# Credits and Acknowledgements

I would like to thank Professor Larry W. Mays for his support and advancing. School of Sustainable Engineering and the Built Environment, Arizona State University, Tempe. E-mail; [Mays@asu.edu](mailto:Mays@asu.edu)

- Civil Engineering department at King Saud University for unlimited support and encouragement.



*Thank You*

# List of References

- Al-A'ama, M. S., and Nakhla, G. F. (1995). Wastewater reuse in Jubail, Saudi Arabia. *Water Research*, 29(6), 1579-1584.
- Alfaisal, F.M., Mays, L.W. Optimization Models for Layout and Pipe Design for Storm Sewer Systems. *Water Resour Manage* 35, 4841-4854 (2021). <https://doi.org/10.1007/s11269-021-02958-5>
- Al-Zahrani, M., Musa, A., and Chowdhury, S. (2016). Multi-objective optimization model for water resource management: a case study for Riyadh, Saudi Arabia. *Environment, Development and Sustainability*, 18(3), 777-798.
- Brand, N., and Ostfeld, A. (2011). Optimal design of regional wastewater pipelines and treatment plant systems. *Water Environment Research*, 83(1), 53-64.
- Cunha, M. C. (2010). Wastewater systems management at the regional level. In Vladimír Olej and I. Obršálová (Eds.), *Environmental Modeling for Sustainable Regional Development: System Approaches and Advanced Methods*
- Haghghi, A., and Bakhshipour, A. E. (2016). Reliability-based layout design of sewage collection systems in flat areas. *Urban Water Journal*, 13(8), 790-802.
- Kajenthira, A., Siddiqi, A., and Anadon, L. D. (2012). A new case for promoting wastewater reuse in Saudi Arabia: Bringing energy into the water equation. *Journal of Environmental Management*, 102, 184-192.
- Karovic, O., and Mays, L. W. (2014). Sewer system design using simulated annealing in Excel. *Water Resources Management*, 28(13), 4551-4565.
- Mays, L. W., Ocanas, G., and Schwartz, M. (1983). *Development and application of models for planning optimal water reuse*. Austin, TX: University of Texas at Austin. Center for Research in Water Resources CRWR-202.
- Mays, L. W. (1976). *Optimal layout and design of storm sewer systems* (Doctoral dissertation, University of Illinois at Urbana-Champaign).
- Mays, L. W. (2001). *Stormwater collection systems design handbook*. New York, NY: McGraw-Hill Professional.
- Mays, L. W. (2019). *Water Resources Engineering*. New York, NY: John Wiley and Sons.
- Mays, L. W., Ocanas, G., and Schwartz, M. (1983). *Development and application of models for planning optimal water reuse*. Austin, TX: University of Texas at Austin. Center for Research in Water Resources CRWR-202.
- Swamee, P. K., and Sharma, A. K. (2008). *Design of water supply pipe networks*. Hoboken, NJ, USA: John Wiley and Sons.